

HYDRO - ELECTRIC DEVELOPMENT
ON THE SNAKE RIVER, OREGON

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ARMOUR INSTITUTE OF TECHNOLOGY

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Hydro-electric development
on the Snake River at Oxen

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HYDRO-ELECTRIC DEVELOPMENT
ON
THE SNAKE RIVER
AT
OXBOW BEND, OREGON
A THESIS

PRESENTED BY

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3

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B I B L I O G R A P H Y.

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Hydro-Electric Plants and Designs of Dams	Beardsley.
Hydraulics,	Dunkersley.
Water Power,	Frizell.
Long Distance Power Transmission	Hutchinson.
Hydro Electric Power Plants	Koester.
Development and Electric Distribution, of Water Power	Lyndon.
Hydro-Electric Practice	Schon'von.
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Alternating Current Machines,	(Sheldon, (Mason & (Hansmann.
Design and Construction of Dams	Wegmann.
Power Plant Engineering	Weingreen.

HYDRO-ELECTRIC DEVELOPMENT ON THE SNAKE RIVER

AT OX BOW BEND, OREGON.

The western part of the United States contains a large percentage of the total water power of this country. That portion, which required little money for development and which was near to a market, has been made use of. The greater part, however, is located at large distances from the populous manufacturing centers.

One of the large undeveloped water powers is at Ox Bow Bend on the Snake River. There is little market for the energy in this locality and hence the development of a hydroelectric plant was considered impractical. However, during the last few years, great strides have been made in high tension power transmission and it is now possible to economically carry power large distances to a market. Thus, the output of a plant at Ox Bow can be transmitted to a market at Boise.

At Ox Bow, the Snake River describes a huge bend which has the shape of an ox yoke. The water is to be backed up at this point to create a head of 52 feet, and delivered to the wheels at the lower end of the bend through a tunnel across the narrow neck of land inside of the bend.

A gate is to be placed at the entrance to the tunnel and a settling basin will be located at the discharge end. The minimum stream flow of the Snake River at this point is 6,500 cubic feet per second, so that the minimum theoretical power will be 38,200 H.P. This is determined from the following equation:

$$\text{H.P.} = \frac{r \ h \ q}{550}$$

where r = weight of 1 cu. ft. of water in lbs. = 62.5

h = head in feet.

q = discharge in cu. ft. per second.

The station has been designed for 37,200 H. P. at the water wheels. It is proposed to install six units, each consisting of two twin turbines direct connected to a 3,750 K.V.A., 60 cycle, three phase alternator. The water wheels which develop 6,200 H. P. at full gate opening are controlled by oil pressure governors. The power factor of the system is estimated at 85%. From the above it is seen that the capacity of the water wheels is about 50% greater than the generator. This is due to the fact that a turbine can not stand the overload which the alternator can. The excitation for the generators is to be furnished by two water wheel driven exciters. Either of these exciters, which are of 300 K.W. capacity, is capable of supplying all the direct

current required by the alternators, control circuits and auxiliary apparatus. The voltage of the exciters is 125. The water wheels are 500 H. P. in capacity and are placed in separate bulk heads. The intake gates are motor operated but in case of emergency can be controlled by hand.

The three phase current from the alternators is stepped up from 2,300 volts to 66,000 volts by three single phase transformers. Both sides of the transformers are connected in delta. Only the high side of the transformers are paralleled since all of the energy is to be sent out of the station at the high voltage. An extra transformer is available for use in case of breakdown. Electrolytic lighting arresters are used to prevent trouble due to storms and other static disturbances.

All the apparatus in the station is controlled from a switch board placed upon the gallery. The board is of the bench type and is very compact.

DAM.

In the design and construction of a hydro-electric project, the dam is of great importance. Upon its stability depends not only the continuous operation of the plant but also the lives and property of the people living the valley below. The financial success of the enterprise, too, depends to a considerable extent upon the amount of money expended in damming up the stream. For these reasons, the dam should be carefully looked into and a type and design selected that will insure maximum safety at the minimum expense.

Of the many types of dams which have been built, the most common form of a permanent type is a solid masonry structure. With an abundance of good stone near at hand and high freight charges on steel, this class of dam will probably be as cheap as the reinforced type which is of lighter section.

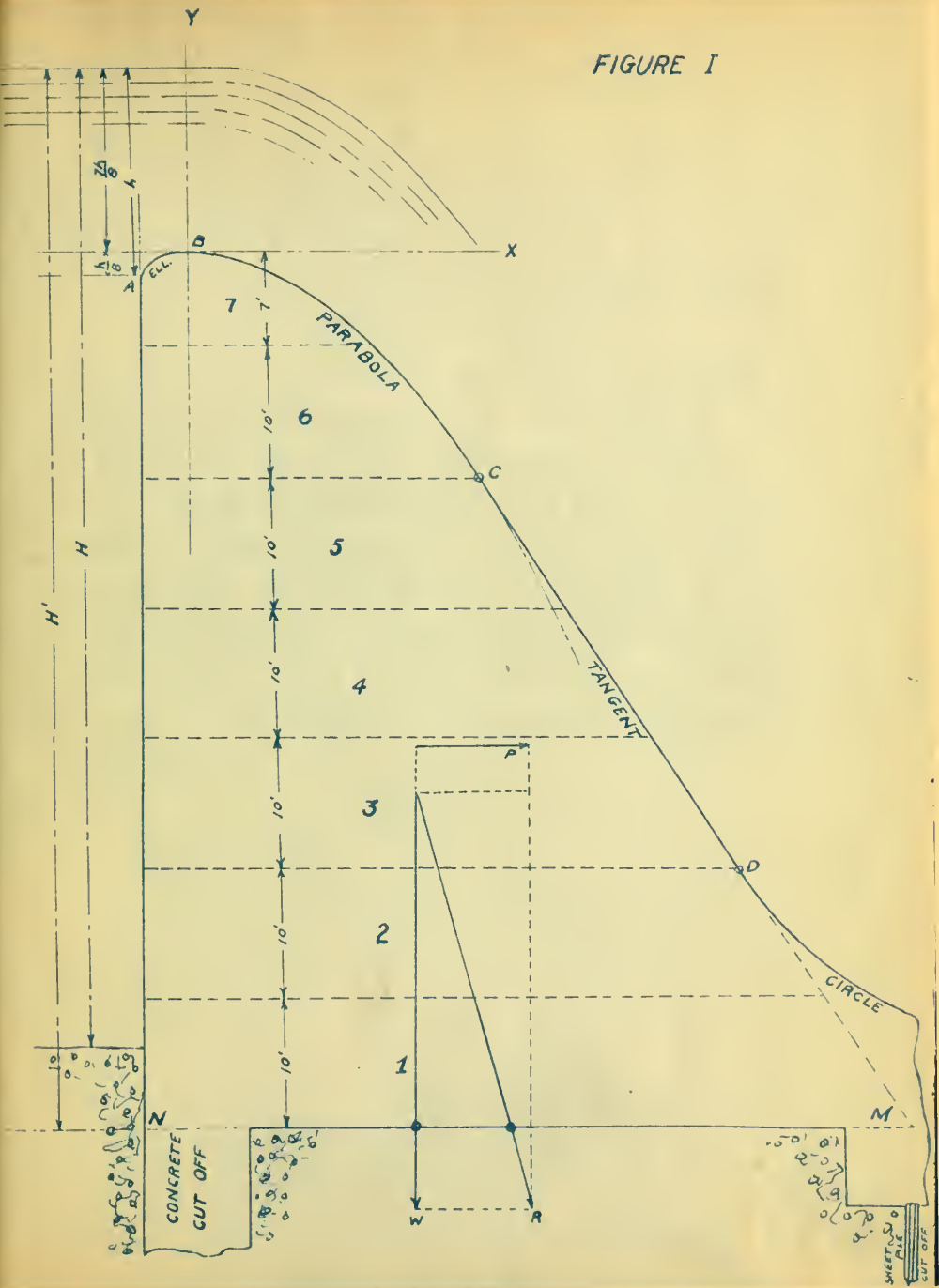
The solid section has proven very reliable, for of all those that have been built, only a few have failed to stand the test of time and floods. Of the failures, most can be attributed to one of two faults in design. Either the dam was overturned because the spillway section was not large enough to take care of an unusual flood, or it was floated away because improper foundation allowed seepage underneath the dam.

To design a dam at Ox Bow Bend it is necessary to know the maximum stream flow of the Snake River at this point. A record of the principal rivers of the United States is kept by the United States Geological Survey. From this record, the maximum flow over a period of years was determined to be about 70,000 cubic feet per second. The maximum amount of water used by the turbines is 6,500 cu. ft. per second. On account of this vast quantity of water to be wasted, it was decided to build the entire dam a spillway. The main spillway which is 393 feet long will take care of the total 70,000 seconds feet, and a shorter spillway at one side will act as a factor of safety in case of an unusual flood. It is essential to look out for floods in the Snake River Valley as the trees which help hold back the spring freshets are fast being removed, due to forest fires and lumbering. An earth retaining section, with a concrete core, has been constructed to prevent seepage at the end of the dam.

With the assumption that the 393 feet of main spillway takes care of the 70,000 cubic feet per second, the water will rise to the height of 14.2 feet above the dam. This is determined by Francis' formula

$$Q = 3.33 Lh^{\frac{3}{2}}$$

FIGURE I



$$h^3 = \frac{Q}{3.33 L} = \frac{70,000}{3.33 \times 393} = 53.5$$

$$h = 14.2$$

Where h is the head of water above the dam, in feet

Q is the flow in cubic feet per second

L is the length of the overflow section, in feet.

To obtain the smallest amount of action on the dam by the falling water and to obtain an extra amount of head in draught times, the section of the spillway should be the natural curve of water running over a sharp edged weir. This section is shown in figure I. Curve AB is an ellipse having a major axis equal to $\frac{h}{2} = 7.1$ feet, and a minor axis equal to $\frac{h}{4} = 3.55'$. Curve BC is a parabola the equation of which is

$$x^2 = 2 py$$

$$\text{where } p = .99 h$$

$$\text{so } x^2 = 2 x .99 x 14.2 y = 28.1 y$$

Solving this equation the following values of X and Y were determined.

X	Y
0	0
5	.89
10	3.56

<u>X</u>	<u>Y</u>
15	8
20	14.2
25	22.2
30	32
35	43.6

The next portion of the curve is a straight line, CD, drawn tangent to the parabola through the point M.

M is one extremity of the base of the dam and it is so located that the length of the base NM is equal to 75% of the maximum head of water above this base.

The last portion of the curve of the section is a circle DE drawn tangent to the line CM. Its purpose is to direct the water horizontally and its radius may be any convenient distance. It was chosen at thirty feet.

As the gravel which forms the river bottom is a layer of very great depth, it would make the dam of enormous cost if the foundation were taken to bed rock. To prevent erosion at the toe of the dam, a very long apron is carried out. As seepage is likely to occur with a gravel foundation two sheet pile cut offs were decided upon, one at the end of the apron and the other at the toe of the dam. A concrete cut off was also placed at the heel.

The stability of vertical wall against water pressure on one side depends upon three things, for proper security.

First, the resultant of the water pressure, acting horizontally through the center of pressure, and the weight of the dam, acting vertically through the center of gravity, must fall within the middle third of the base.

Second, the maximum pressure per cent area on the base must not exceed a safe value.

Third, the water pressure must be less than the sliding friction on the base.

In a dam of the solid type, the weight of the dam, and the area of the base must be of such a magnitude, if the first condition holds, that the third condition will be absolutely safe, so that it is unnecessary to investigate the strength against sliding.

To determine the line of pressure in the dam the following method was used. The dam was first divided into seven sections, six of which were ten feet each, and the seventh was 7.2 feet high. The center of gravity of the dam was then determined, first for the whole section, then with section 1 removed, then with sections 1 and 2 removed and so

on until only section 7 remained. The line of pressure with no water in the reservoir can be determined by projecting the centers of gravity for the different sections upon the base line of those sections and then connecting points. It was found that the resultant line of pressure lay well within the middle third of the dam.

To determine the line of pressure with any water behind the dam it is necessary first to determine the weights of the different sections. This was done by drawing the section to scale and measuring the area with a planimeter. The following results were obtained with the dam drawn to a scale of 1 inch equal to ten feet.

Section	Area sq. in. (by planimeter)	Vol. of Sect. 1 ft. wide. (cu.ft)	Wt.of Sect. 1 ft. wide #.
1-7	24.12	2412	361,500
2-7	18.68	1868	280,200
3-7	13.86	1386	207,900
4-7	9.53	953	142,950
5-7	6.05	605	60,750
6-7	3.12	312	46,800
7	.93	93	13,950

The material of the dam which is granite was assumed to weigh 150 pounds per cubic foot. The forces shown in the fourth column represent the weights of the various sections acting down through the centers of gravity of the respective sections. It is now necessary to obtain the force and the point of application of the force due to the water. The center of pressure, X_C , was determined by the equation

$$X_C = \frac{\frac{2}{3} h_2^3 - h_1^3}{h_2^2 - h_1^2}$$

The force, P , due to the water acting at X_C , was determined by the equation

$$P = \frac{1}{2} b h h_3 r$$

r is the weight of one cubic foot of water and is equal to 62.5^{lb}, b is the width of the section and was taken as one foot. The remaining terms are explained by figure II.

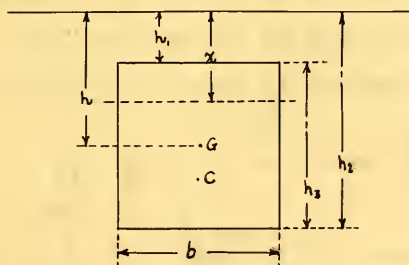


Fig. II.

The following results were obtained.

Sect.	h_1	h_2	X_c	h	h_3	P
1-7	14.2	75.2	51.6	44.5	61	84,800
2-7	14.2	71.4	49.1	42.6	57.2	76,100
3-7	14.2	61.4	42.6	37.6	47.2	56,700
4-7	14.2	51.4	36.2	32.6	37.2	36,700
5-7	14.2	41.4	30	27.6	27.2	23,500
6-7	14.2	31.4	23.8	22.6	17.2	12,150
7	14.2	21.4	18.0	17.6	7.2	3,960

By combining P acting horizontally thru C and the weight of the dam acting vertically thru the center of gravity as previously determined, the resultant pressure may be determined for each section. By connecting the points where this pressure acts on the bases of the respective sections, the line of resultant pressures may be obtained. This line was found to be very close to the center of the dam.

It is now necessary to determine the maximum pressure acting on the base of the dam.

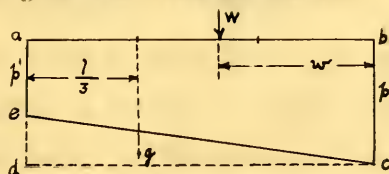


Fig. III.

Figure III represents the trapezoid of pressure on the base of the dam.

W = the total pressure acting on ab

w = the distance of W from the nearer edge, b

p = the maximum intensity of pressure

p^1 = the minimum intensity of pressure

b = the width of the base ab

g = the C.G. of the triangle Ced .

Trapezoid $abce$ = the reaction of the foundation

$$= abcd - ced$$

$$= p^1 b - \frac{1}{2} (p - p^1) b$$

Since W and the reaction of the foundation are in equilibrium, the algebraic sum of their moments about any point must be equal to zero. Taking moments about, we find the moment of $ced = 0$.

$$\frac{1}{2} p b - W \left(\frac{2}{3} b - w \right) = 0$$

$$p = \frac{2w}{b} \left(2 - \frac{2w}{b} \right)$$

$$p = 361,500 \quad b = 58.8 \quad w = 28.4$$

$$p = \frac{2 \times 361,500}{58.8} \left(2 - \frac{2 \times 28.4}{58.8} \right)$$

$$= 12300 \times .55 = 6765 \text{ per sq. ft.}$$

This value is within the safe limit for coarse gravel which forms the foundation of the dam.

It was previously determined that the line of pressure, with the reservoir both empty and full, was well within the middle third of the dam. As the unit pressure is found to be within the safe working limit of the material of the foundation, the dam will be safe in every respect.

TRANSMISSION LINE.

The line is one of the most important parts of the system, for on it depends the continuity of service without which the best apparatus is useless. On this account it is important to consider carefully not only electrical details, but also methods of construction.

In the design of a transmission system, the line must be considered, first, in its general relations to the plant, second, as a special problem in engineering, third, as a mechanical structure. The first has to do with the proper proportioning of the line as a part of the system, its function as a distributing conductor, and its bearing on the general efficiency of the plant of which it is a part. In connection with this, it must be remembered that much depends on the type and adjustment of the station apparatus in controlling the line voltage, loss and power factor. The second deals with the electrical difficulties, and the third with the materials of construction and methods of applying them.

In the passing of energy over a line there is first the loss due to resistance. The two conductors with the air between them act as a condenser.

This condenser action tends to cause the generator voltage to rise, while the inductance of the line has the opposite effect.

The charging current is constant and, flows as long as voltage is applied to the line. Its importance diminishes as the load increases for when the load is heavy, it is not only overcome by the lagging current of the load, but is also rendered unimportant by the presence of a large current in place with the e. m. f.

In the case of large excess lagging current, the effect on the capacity and regulation of the generator is well known. The lagging load current can be balanced by synchronous machines. Where synchronous machines are not used, a large line capacity current may be desirable. The capacity may be increased by dividing the conductors into two or more wires, separated from each other, but mounted on the same insulator or adjacent insulators. This will at the same time decrease the inductance.

The line which it is proposed to construct in connection with the generating station at Ox Bow Bend will be approximately one hundred and seventy-five miles long.

The voltage has been fixed at 66000 since this is the standard in this locality and it is probable that in the future this station will be tied in with some of the existing plants.

The main line will be run in duplicate on steel towers to insure continuity of service. Three-phase transmission has been decided upon, since it is the most economical in the amount of line wire required. The line side of the transformers will be connected in delta to prevent trouble from harmonics.

The location and capacity of the several substations have been determined from estimations made on the amount of power which can be sold in the several towns. There will be eight substations ranging in capacity from 150 K.W. to 5000 K.W. It is proposed to construct a special line six miles in length to a mine which will require 3000 K.W. This line will be of similar construction to the main line, and will be operated at 66000 volts, for it is believed that this line will ultimately be extended. Two short lines from Boise will be operated at 66000 volts for this same reason. The substations on these lines are small, and the income from them will not justify a steel tower construction with duplicate lines. Wooden poles will be used, and only one line constructed.

TRANSMISSION LINE.

Section 1. Oxbow to Weiser.

The power to be transmitted at present is 7400 K.W., but we shall design the line to carry double this amount to allow for future growth.

Length of section - 65 miles.

Current to be carried at an assumed power factor of 85%.

$$I = \frac{15,000,000}{\sqrt{3} \times 66,000 \times .85}$$
$$= 155 \text{ amperes.}$$

For this section we shall use #1 copper B. & S. equivalent aluminum standard cable.

Area in circular miles = 132,300

Diameter = .42 inch

Area = .1039 square inch

Weight per mile = 642.6 pounds

Resistance per mile to 60 cycle current = .7034 ohms

Elastic limit = 1450 pounds.

Inductance per wire per mile = $(805 + 740 \log \frac{d}{R}) \times 10^{-6}$

d = distance between wires = $(805 + 740 \times 2.6) \times 10^{-6}$
= 84 inches = .002 henry.

R = radius of wire

= .21 inch.

$$\text{Capacity between conductors per mile} = \frac{.0388}{2 \log \frac{d}{R}} = .0075 \text{ m.f.}$$

A convenient method of calculating the regulation of a three-phase circuit is to consider it as two single phase circuits, having the same size conductors, same voltage and spacing, and each carrying one half of the total power transmitted.

We shall consider this portion of the line as two single phase lines, each transmitting 3750 K.W.

$$\text{Inductance from Oxbow to Weiser} = 65 \times 2 \times .002$$

$$= .26 \text{ henry}$$

$$\text{Reactance} \quad wL = 2 \pi \times 60 \times .26$$

$$= 92 \text{ ohms.}$$

$$\text{Capacity between lines from Oxbow to Weiser} = 65 \times .0075$$

$$= .49 \text{ microfarad.}$$

$$WC = 377 \times .49 \times 10^{-6}$$

$$= .000185$$

$$\text{Resistance of circuit} = 65 \times 2 \times .703$$

$$= 91.5 \text{ ohms.}$$

In order to calculate the regulation, we shall divide the line into ten sections.

For each section:

$$L = .026 \text{ henry} \quad \omega L = 9.2 \text{ ohms.}$$

$$C = .049 \text{ microfarad} \quad \omega C = .0000185$$

$$R = 9.15 \text{ ohms.}$$

Let us assume the drop is 10%

Voltage at Oxbow = 66,000

Voltage at Weiser = 59,400.

At present we shall confine ourselves to the re-regulation of the line when carrying the present load.

(7400 K.W. or 3700 K.W. on each single phase line.)

Current in section 1 = 73 amperes.

Resistance drop in section 1, R.I. = $9.15 \times 73 = 668$ volts.

Inductive drop in section 1, X.I. = $9.2 \times 73 = 672$ volts.

Voltage at end of section 1, $E_1 = \sqrt{(59400 + 668)^2 + (672)^2}$.
= 60072 volts.

Effect of charging current.

The maximum charging current will be that in section 10 where the voltage is 66,000.

$$\begin{aligned} I_C &= 2 \pi f C E. \\ &= .0000185 \times 66,000 \\ &= 1.22. \end{aligned}$$

$$\begin{aligned} \text{Resultant current} &= \sqrt{(73)^2 + (1.22)^2} \\ &= 73.09. \end{aligned}$$

The error in neglecting the effect of charging current will be $\frac{.09}{73} \times 100 = .12$ of one per cent.

Hence we will neglect the charging current.

Since we have decided to neglect the charging current, the resistance and reactive drops will be the same in all sections.

$$E_2 = \sqrt{(60072 + 668)^2 + (672)^2}$$

$$= 60743 \text{ volts.}$$

$$E_3 = \sqrt{(60743 + 668)^2 + (672)^2}$$

$$= 61413 \text{ volts.}$$

$$E_4 = \sqrt{(61413 + 668)^2 + (672)^2}$$

$$= 62083 \text{ volts.}$$

$$E_5 = \sqrt{(62083 + 668)^2 + (672)^2}$$

$$= 62753 \text{ volts.}$$

$$E_6 = \sqrt{(62753 + 668)^2 + (672)^2}$$

$$= 63424 \text{ volts.}$$

$$E_7 = \sqrt{(63424 + 668)^2 + (672)^2}$$

$$= 64095 \text{ volts.}$$

$$E_8 = \sqrt{(64095 + 668)^2 + (672)^2}$$

$$= 64765 \text{ volts.}$$

$$E_9 = 65346 \text{ volts.}$$

$$E_{10} = \sqrt{(65346 + 668)^2 + (672)^2}$$

$$= 66107 \text{ volts.}$$

This is within .16 of one per cent, hence the drop assumed is satisfactory.

THE LINE AS A MECHANICAL STRUCTURE.

Weight per foot of cable = .121#

Area of conductor = .104 square inch

Diameter of conductor = .42 inch

Elastic limit of cable = 1450#.

We shall assume that the line is covered with 1/2 inch of ice, and that the wind pressure is 15# per square foot.

S_1 = span in feet = 500

D = sag in feet

W = weight of conductor and ice in pounds per foot

W_2 = resultant of W and wind pressure in pounds per foot

T = maximum allowable tension in conductor = 1450#

K = temperature coefficient of linear expansion per degree Fahrenheit = .0000128

t = temperature in degrees Fahrenheit above minimum (-40°) = 125.

E_1 = stretch modulus of elasticity = 9,000,000.

L_s = length of single span of strung cable at minimum temperature, in feet.

L_u = length of unstressed single span of cable at minimum temperature.

$$L_s = S_1 + \frac{S_1^3}{24} \frac{W_r^2}{T^2}$$

$$L_u = \frac{L_s}{1 + \frac{T}{AE_1}}$$

$$D^3 - \frac{3}{8} S_1 (L_u(1+Kt) - S_1) D = \frac{3 S_1^3 L_u W_r}{64 AE_1}$$

$$D^1 = \frac{DW}{W_r} \text{ vertical sag.}$$

Weight of ice per cubic foot = 57#.

$$\text{Weight of } 1/2 \text{ inch of ice} = \frac{12 \pi ((1.42)^2 - (.42)^2) \times 57}{1728}$$

$$= .57\# \text{ per foot of cable.}$$

Weight of cable and ice = .691# per foot.

$$\text{Wind pressure on ice covered cable} = \frac{1.42 \times 12 \times 15}{144}$$

$$= 1.78\# \text{ per foot of cable.}$$

$$\text{Resultant, } W_R = \sqrt{(.691)^2 + (1.78)^2}$$

$$= 1.91\# \text{ per foot of cable.}$$

$$L_s = 500 + \frac{(500)^3 \times (1.91)^2}{24 \times (1450)^2}$$

$$= 509.04 \text{ feet.}$$

$$L_u = \frac{509.04}{1 + \frac{1450}{9,000,000 \times .104}}$$

$$= 508.25 \text{ feet.}$$

$$D^3 - \frac{3 \times 500}{8} (508.25 \times 1.0016 - 500) D = \frac{3 \times (500)^3 \times 508.25 \times 1}{64 \times .104 \times 9000000}.$$

$$D^3 - 1690 D = 6080.$$

$$D = 42.8 \text{ feet.}$$

$$D^1 = \text{vertical sag} = \frac{42.8 \times .691}{1.91}$$

$$= 15-1/2 \text{ feet.}$$

The lowest portion of the cable must be a sufficient height above the ground to prevent accidental contact. This distance should be 20 or 25 feet. We shall use a tower having the lower cross arm 40 feet from the ground. The minimum clearance will then be 24-1/2 feet.

Forces acting on the Towers.

The main line from Oxbow to Boise will be built in duplicate. Two three-phase lines arranged in delta, one on each side of the tower will be strung.

$$\text{Weight of conductors and ice} = 6 \times 500 \times .691$$

$$= 2075\#$$

$$\text{Force due to wind on conductors} = 6 \times 500 \times 1.78$$

$$= 5350\#.$$

Surface of tower exposed to wind will be about 30 square feet.

$$\text{Wind pressure on tower} = 30 \times 15.$$

$$= 450\#.$$

This acts at the center of gravity of the tower, but let us assume that 300# acts along the line of the wind pressure on the conductors. Wind pressure acts through the center of gravity of the delta, about two feet above the lower conductors, or 42 feet from the ground.

Total force due to wind = 5350# + 300#

= 5650#.

The towers must support a weight of 2075# and a force of 5650# acting 42 feet from the ground.

SUMMARY.

Length of line - 65 miles.

Transmitted power - 7500 K.W.

Conductors - #1 Equivalent Aluminum Stranded Cable.

Length of span - 500 feet.

Spacing - 7 feet.

Maximum sag - 15-1/2 feet.

Approximate number of towers = $\frac{5280 \times 65}{500}$

= 690

Approximate number insulators = 690 x 6.

= 4140.

TRANSMISSION LINE.

Section 2, Weiser to Horseshoe Bend.

On this line are located four substations, ranging in capacity from 150 K. W. to 600 K. W. We shall consider this section as carrying 6800 K. W., since the substations are small, and the difference between the currents in the several sections, will be slight. Length of line - 65 miles.

The power to be transmitted is 6800 K. W.

$$I = \frac{6800000}{\sqrt{3} \times 59400 \times .85} \quad 59400 = \text{voltage at Weiser.}$$
$$= 78 \text{ amperes.}$$

To allow for a growth of load, we shall use #1 copper B. & S. equivalent aluminum standard cable. Sine constants are the same as for section 1.

Let us assume a drop of 10%.

$$\text{Voltage at Horseshoe Bend} = 59400 \text{ volts.} \times .9 = 53460$$

The method of calculation followed was that used in calculating section 1.

A ten per cent drop is satisfactory.

We shall use the same span and towers as used on section 1.

SUMMARY.

Length of line - 65 miles.

Transmitted power - 6800 K. W.

Conductors - #1 Equivalent Aluminum Standard Cable.

Length of span - 500 feet.

Spacing - 7 feet.

Maximum sag - $15\frac{1}{2}$ feet.

Weight to be supported by towers 2075#.

Force acting due to wind = 5650#.

Arm of above force = 42 feet.

Approximate number of towers = $\frac{5280 \times 65}{500}$

=690.

Approximate number of insulators = 690 x 6.

= 4140.

TRANSMISSION LINE.

Section 3. Horseshoe Bend to Boise.

The section of the transmission line from Horseshoe Bend to Boise is 26 miles long. There are no substations between Horseshoe Bend and Boise. The power to be transmitted at present is 5450 K. W.

We have found that at full load, the voltage will have fallen to 53460 at Horseshoe Bend. We shall assume 60,000 volts at Boise, then calculate the voltage at Horseshoe Bend. Regulators will be installed at Horseshoe Bend to raise the line voltage to that required.

The Horseshoe Bend plant at present is of 1500 K. W. capacity. This plant will be operated in parallel with the Oxbow Bend station.

$$\begin{aligned} \text{line current, } I &= \frac{5450000}{\sqrt{3} \times 60000 \times .85} \\ &= 61 \text{ amperes.} \end{aligned}$$

We shall use #2 equivalent aluminum cable, which will allow for an increase in load of about 75%.

Line Constants.

Inductance per wire per mile = .00197 henrg.

Capacity between conductors per mile = .0073 microfarad.

Resistance per mile per wire = .8873 ohm.

Area of cable = .0824 square inch.

Diameter of cable = .37 inch

Elastic limit = 1155#

Weight per foot of cable = .0965#

The result of the calculation of the voltage required at Horseshoe Bend was 62700 volts. Hence the regulator at Horseshoe Bend must raise the voltage from 53460 volts to 62700 volts.

This section of the line is to be run in duplicate, as were sections 1 and 2. We shall use a span of 500 feet.

The results of the calculations are shown below in the summary.

SUMMARY.

Length of line - 26 miles.

Transmitted power - 5450 K. W.

Conductors - #2 equivalent Aluminum Standard Cable.

Length of span - 500 feet.

Maximum sag - 13 feet.

Weight to be supported by towers - 1910#.

Force acting due to wind - 5450#.

Arm of above force - 42 feet.

Approximate number of towers = $\frac{5280 \times 26}{500}$ -
= 275.

Approximate number of insulators = $275 \times 6 = 1650$.

TRANSMISSION LINE.

Section 4, Boise to Meridian.

A ten mile branch line is to be constructed from Boise to Meridian. At present the power to be transmitted is only 300 K. W. The line current at 60000 volts is so small that the size of conductors used depends on the mechanical strength required.

We shall use #4 B & S. hard drawn copper wire.

Breaking stress - $137\frac{1}{2}$ lbs.

Diameter - .204 inches.

Weight per foot - $.126\frac{1}{2}$ lbs.

Resistance per 1000 feet - .3172 ohms.

Inductance per wire per mile - .0024 henry.

Capacity between conductors per mile - .00665 microfarads.

The result of the calculation of the voltage at Meridian at full load is 53894 volts.

Line Construction.

On this part of the line we shall only run one three-phase line, one conductor on top of the poles, other two on cross arms. The results of the calculations are given below in the summary.

SUMMARY.

Length of line - 10 miles.

Transmitted power - 300 K. W.

Conductors - #4 copper B. & S. hard drawn wire.

Length of span - 75 feet.

Spacing - 7 feet.

Maximum sag - 1.29 feet.

Weight to be supported by poles - 125#.

Force acting due to wind - 488#.

Arm of above force - 25'.

The size of pole required will be a 26 foot white pine pole at least 2 feet in diameter at the butt.

$$\text{Approximate number of poles} = \frac{5280 \times 10}{75}$$

$$= 700.$$

$$\text{Approximate number of insulators} = 700 \times 3.$$

$$= 2100.$$

TRANSMISSION LINE:

Section 5. Boise to Neal.

The distance from Boise to Neal is twenty-one miles. The size of conductors are determined by the mechanical strength required as on section 4. The power to be transmitted is 150 K. W.

We shall use #4 copper B. & S. hard drawn wire. The line constants are the same as for section 4. Line construction is the same as for section 4.

SUMMARY.

Length of line - 21 miles.

Transmitted power - 150 K. W.

Conductors #4 Copper B. & S. hard drawn wire.

Length of span - 75 feet.

Spacing - 7 feet.

Maximum sag - 1.29 feet.

Weight to be supported by poles - 125#.

Force acting due to wind - 488#.

Arm of above force - 25 feet.

The poles to be used are as on section 4.

$$\text{Approximate number of poles} = \frac{5280 \times 21}{75}$$

$$= 1470.$$

$$\text{Approximate number of insulators} = 1470 \times 3.$$

$$= 5410.$$

TRANSMISSION LINE.

Section 6. Branch from Oxbow to Mine.

This line is to transmit 3000 K. W. a distance of six miles. We have decided to run two 66000 volt lines as on the main line. The intention is to ultimately extend this line hence we are using a high voltage line and steel tower construction.

$$\text{Line current } I = \frac{3000,000}{18 \times 66000 \times .85}$$

$$= 31 \text{ amperes.}$$

We shall use #2 equivalent aluminum cables.

The line constants are as for section 3.

The line construction is the same as on section 3.

The voltage at the mine at full load will be 65400.

SUMMARY.

Length of line - 6 miles.

Transmitted power - 3000 K. W.:

Conductor - #2 equivalent aluminum cable.

Length of span - 500 feet.

Spacing - 7 feet.

Maximum sag - 18 feet.

Towers - 40 feet to lowest insulator.

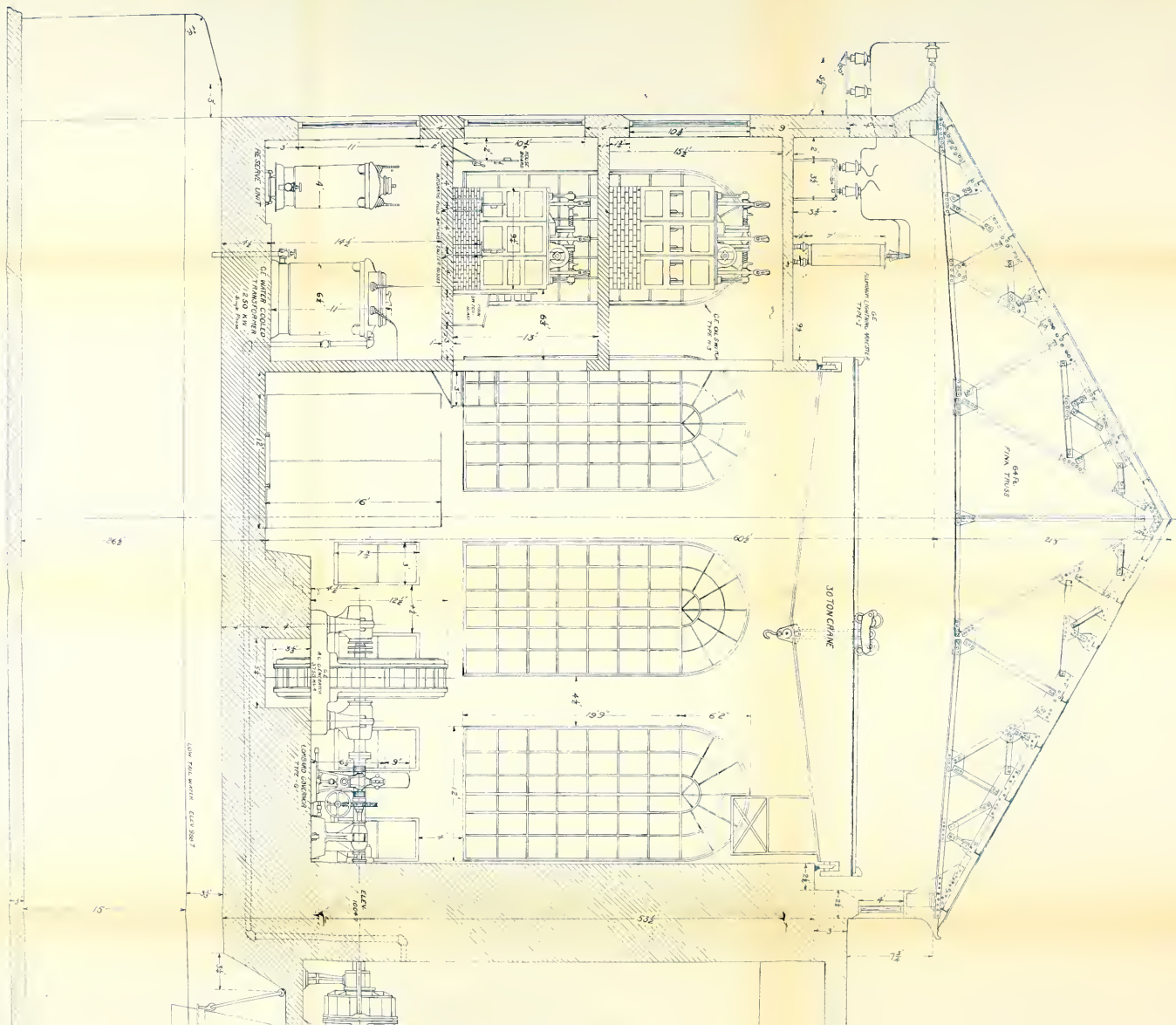
Weight to be supported by towers - 1910#.

Force acting due to wind - 5450#.

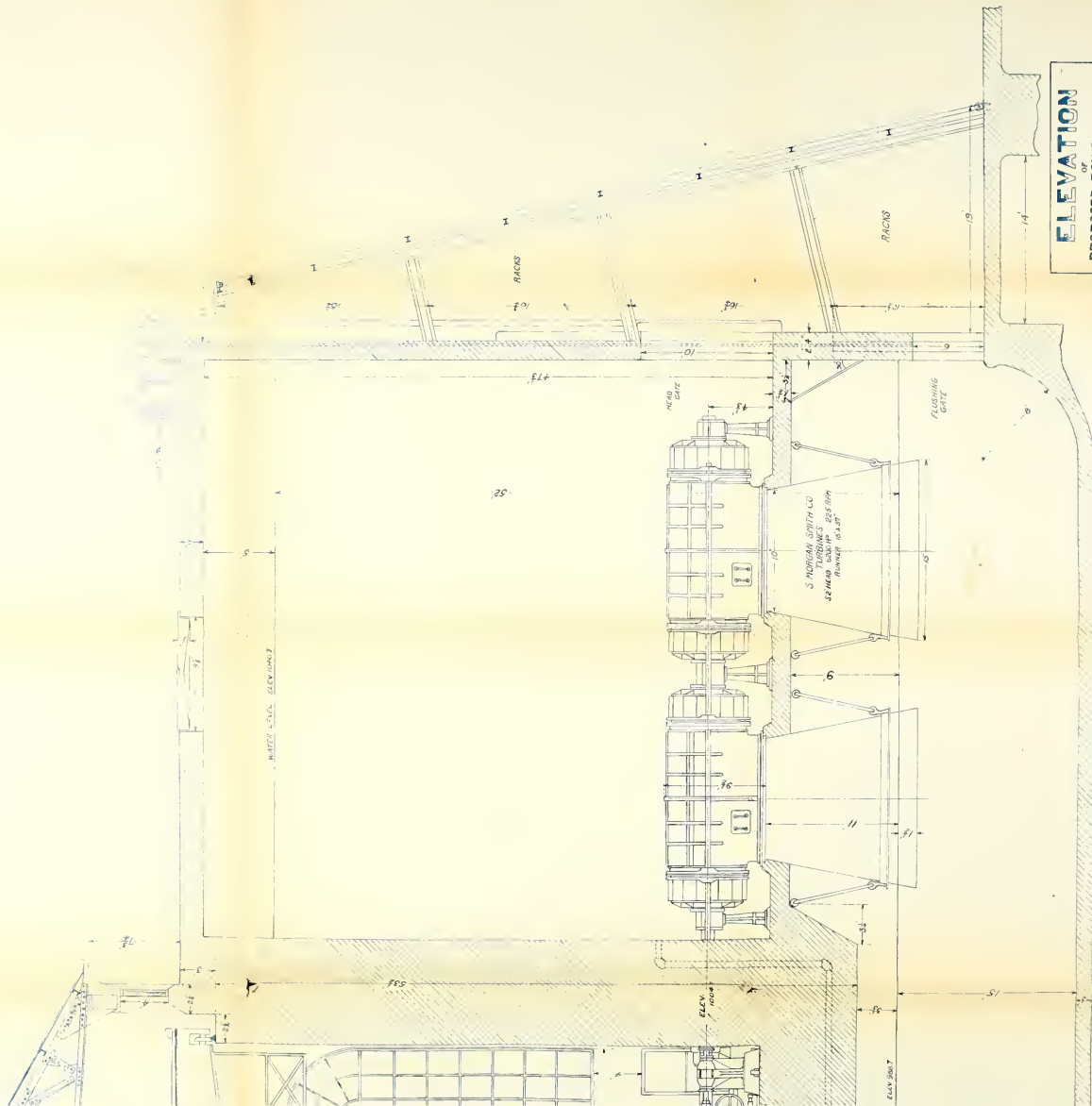
Arm of above force - 42 feet.

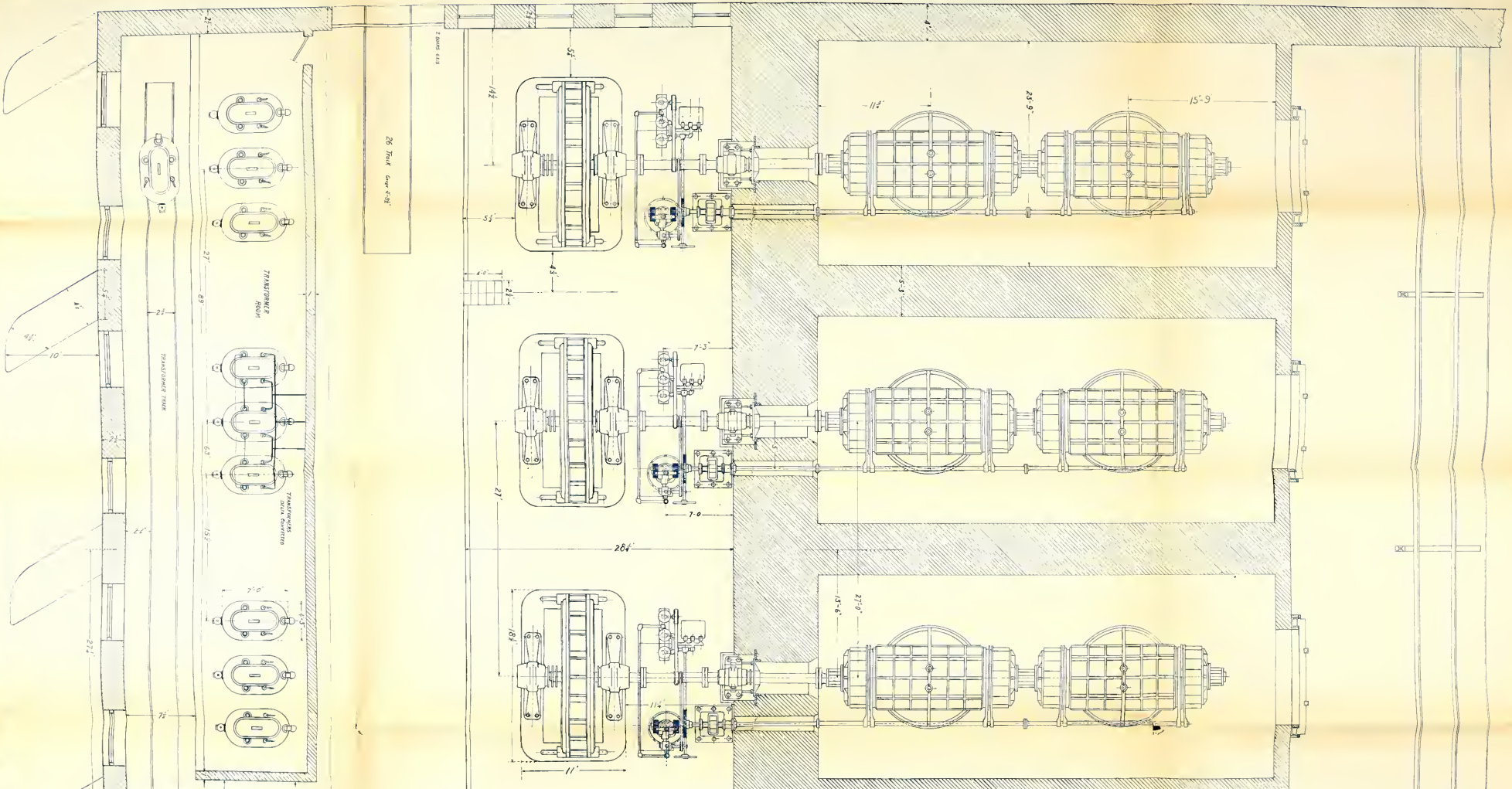
$$\text{Approximate number of towers} = \frac{5280 \times 6}{500} = 65$$

$$\text{Approximate number of insulators} = 65 \times 6 = 390.$$



ELEVATION
OF
PROPOSED POWER PLANT
AT
9X80W, BAKER CO., OREGON
ON THE SNAKE RIVER







6200 MP 52'

TURBINES

3750-KKA

GENERAL ELECTRIC
NOTICE (continued)

SPIRAL STAIRING
DUM-5f

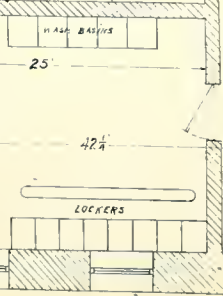
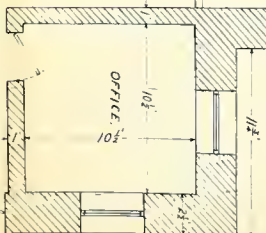
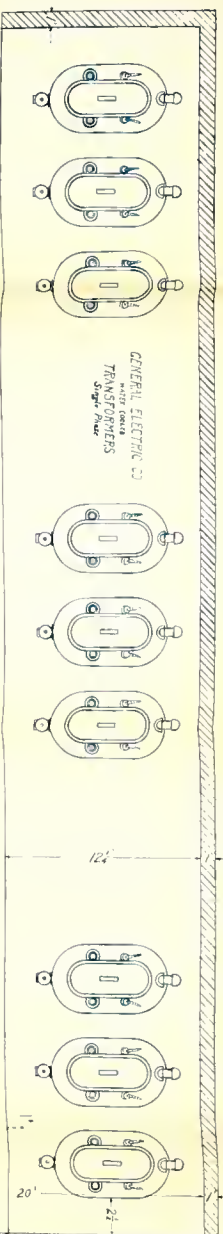
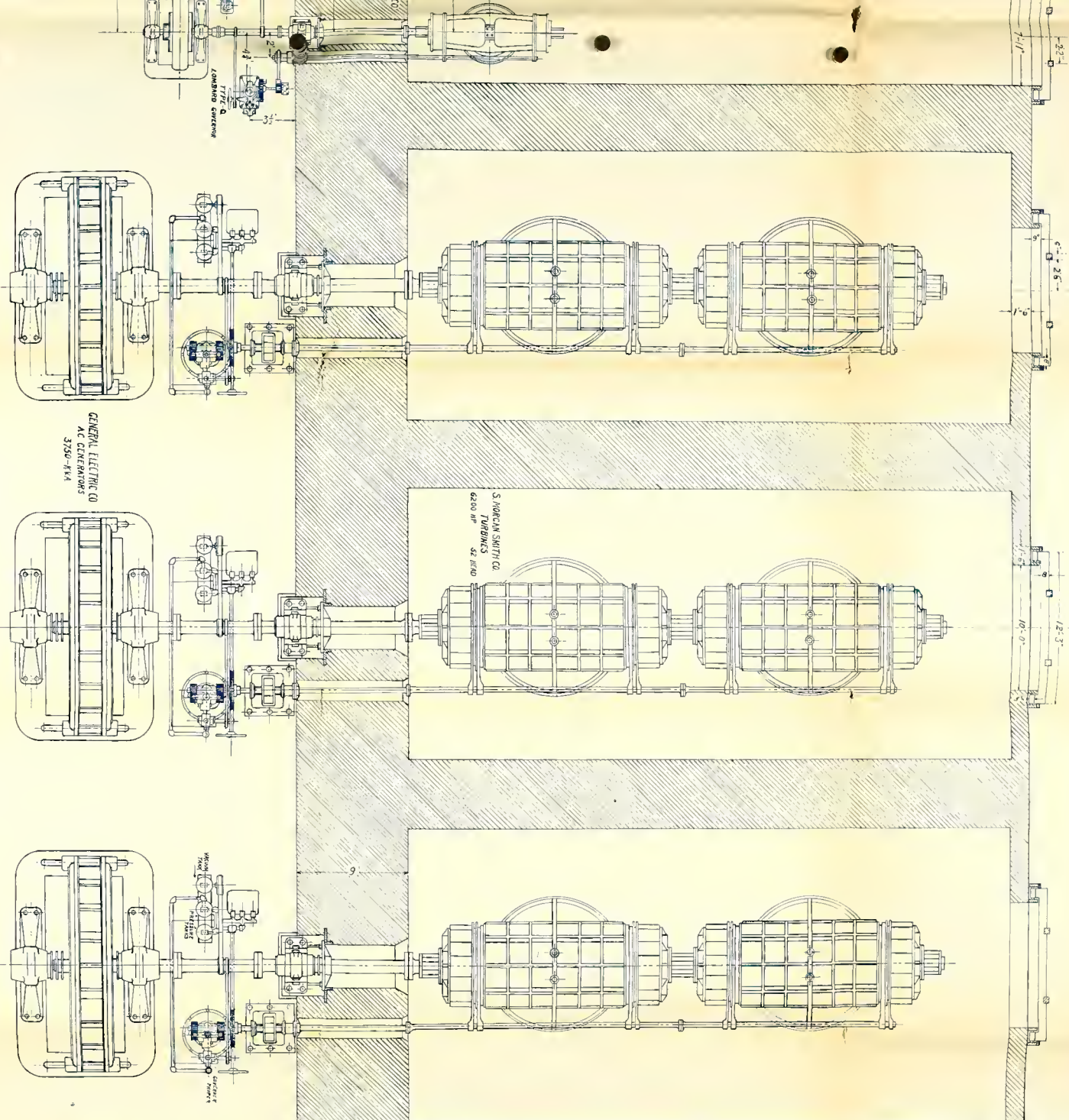
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—15—0—

 51.8°

ROCKS
SECTION FROM A-A



PLAN
OF
PROPOSED POWER PLANT
AT
BARROW, BAKER CO., ALASKA
ON THE SNARE RIVER
MAY 12, 1920

AMERICAN
BUREAU OF ENGINEERING
LITHOGRAPH

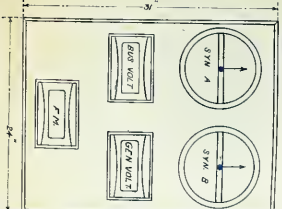
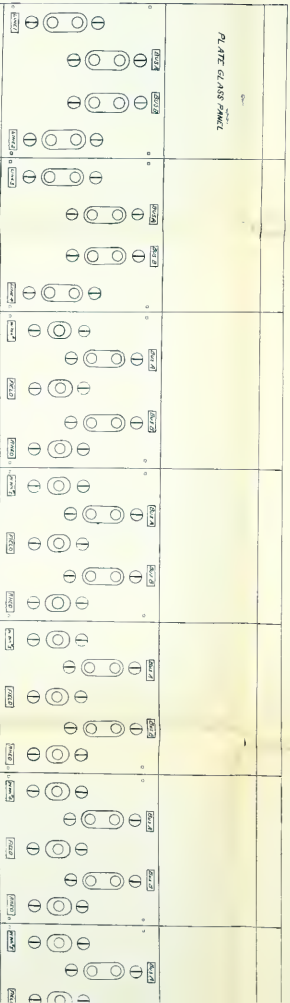
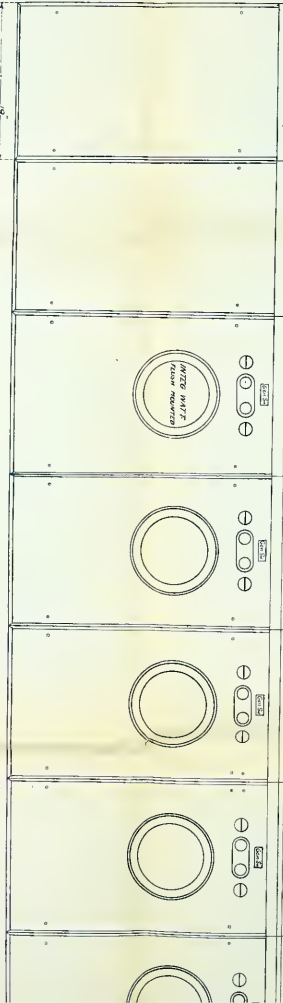
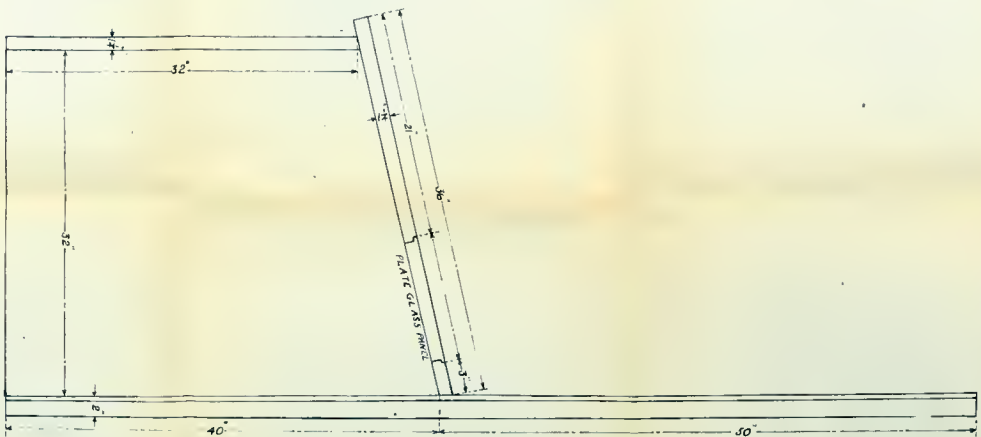


PLATE CLAS PACK
PLATE BOARD
PLATE BOARD





SCALE 2"=1'

MAY 13, 1910

MAY 13, 1910

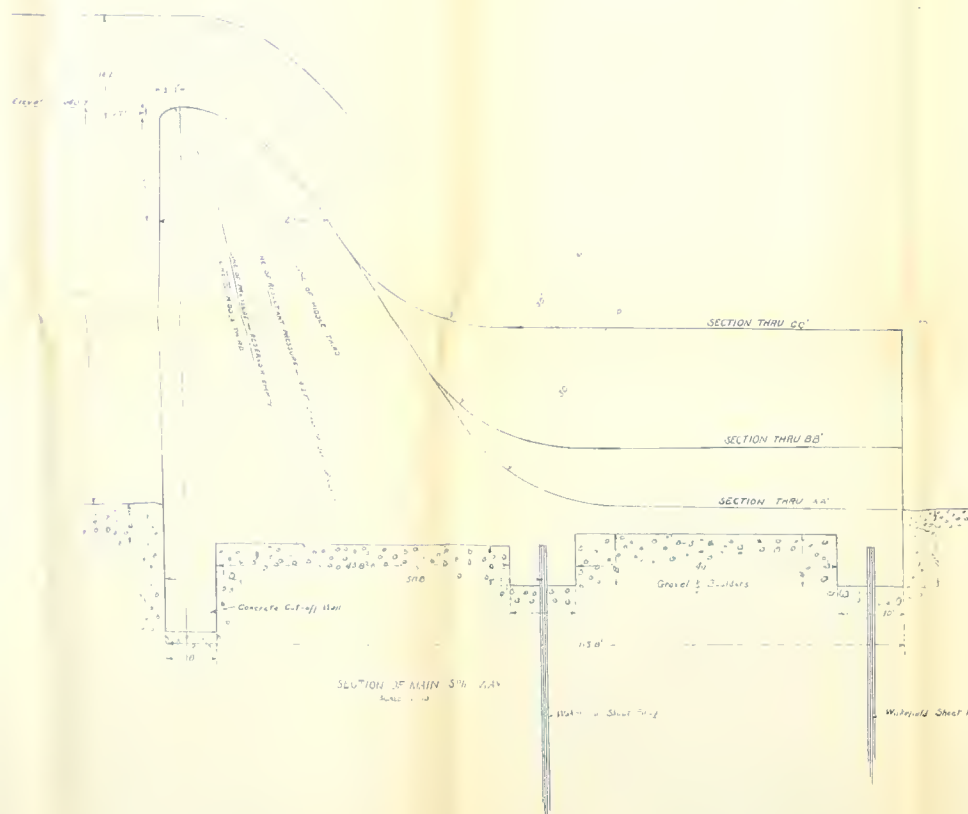
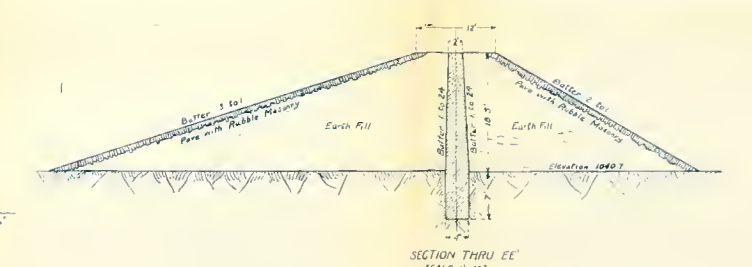
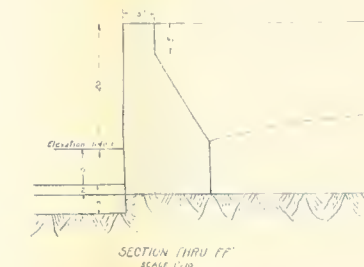
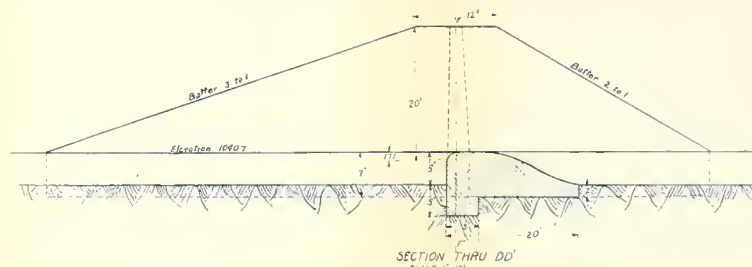
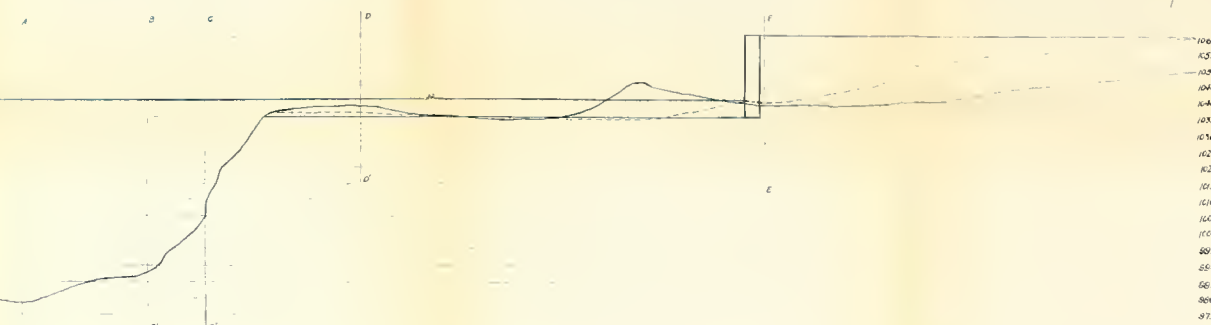
PLAN & TOPOGRAPHY

SCALE 1" = 20'



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PROFILE SCALE 1"=20'



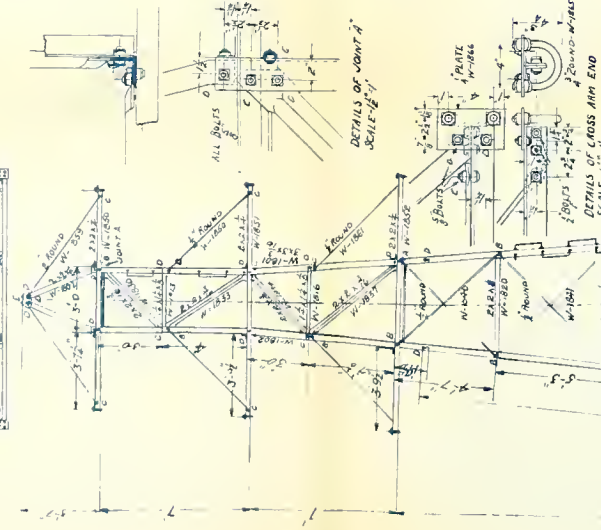
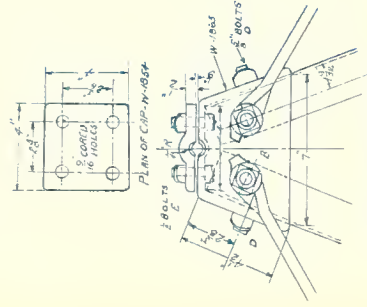
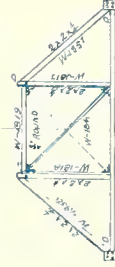
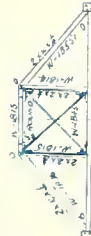
DAM
FOR
PROPOSED POWER PLANT
AT
OXBOW, BAKER CO., OREGON
ON THE SNAKE RIVER
SCALE 43 IND MAY 15, 1910



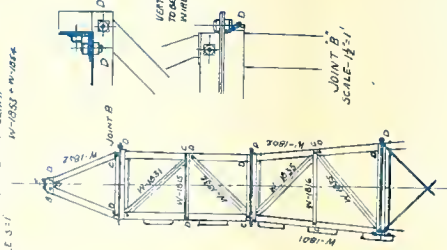
5NAKE



CONTOUR MAP
OF
THE SNAKE RIVER VALLEY
IN THE VICINITY OF
OXBOW BEND
MAY 1930
SCALE 1:100



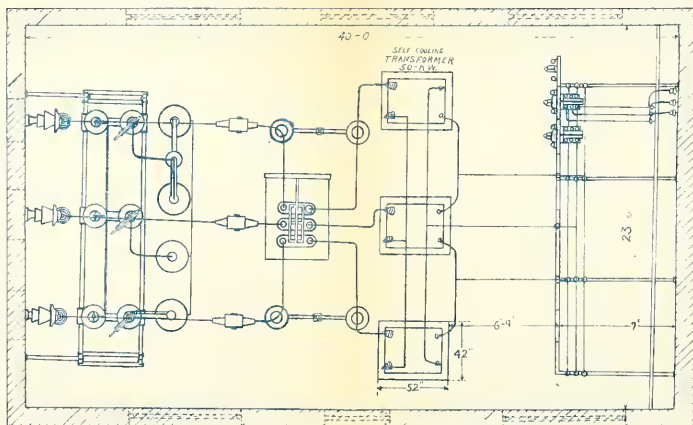
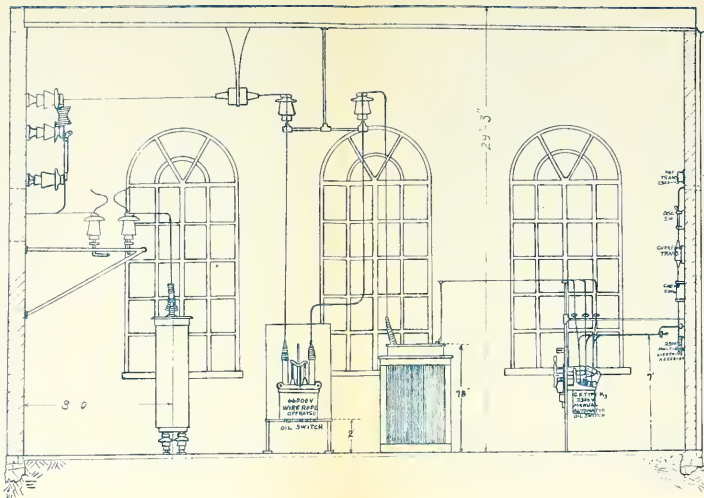
DETAILS OF GROUND ANGLE CLAMP
SCALE 1/2" = 1'



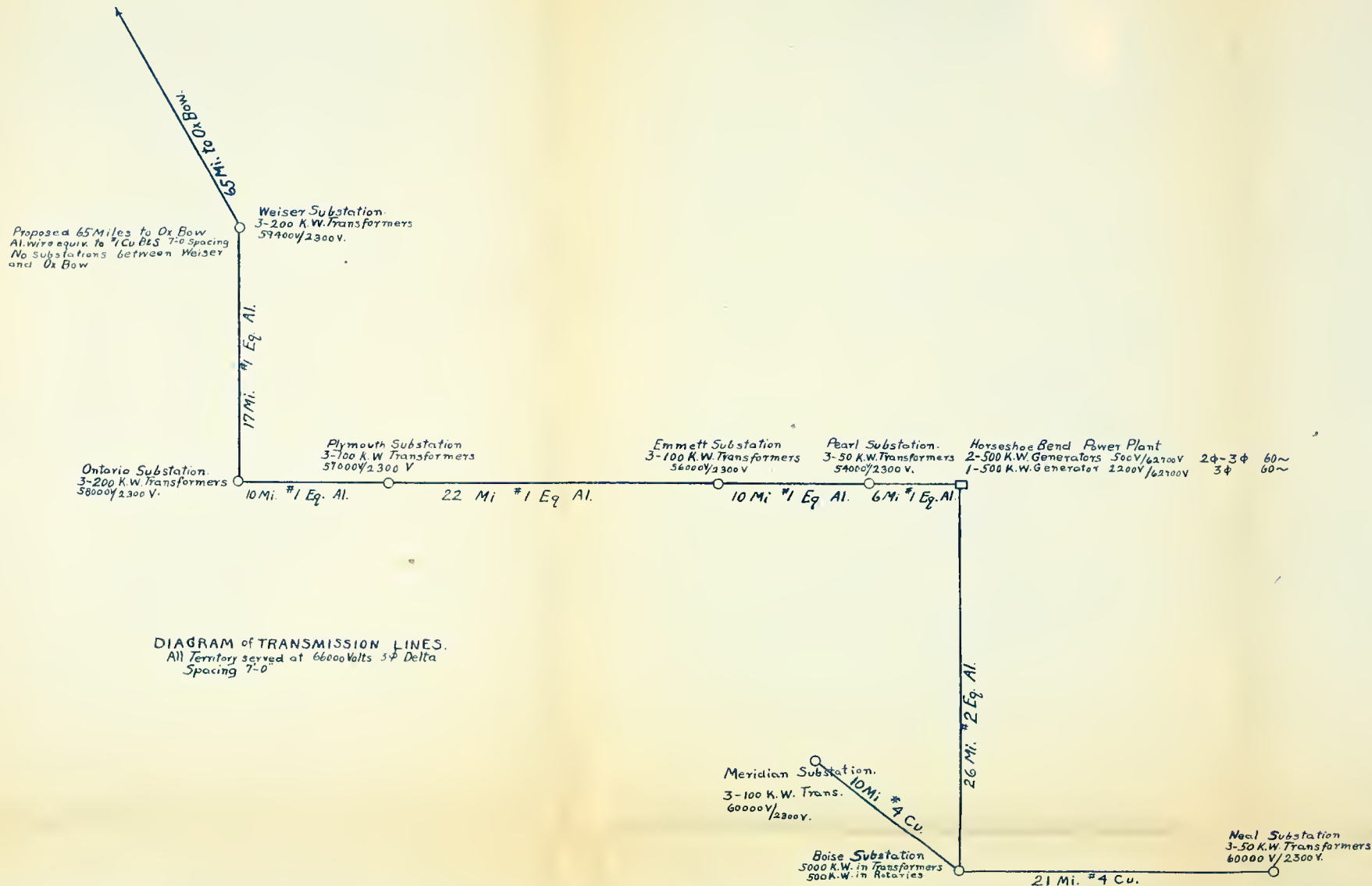
DETAILS OF JOINT

DETAILS OF JOINT

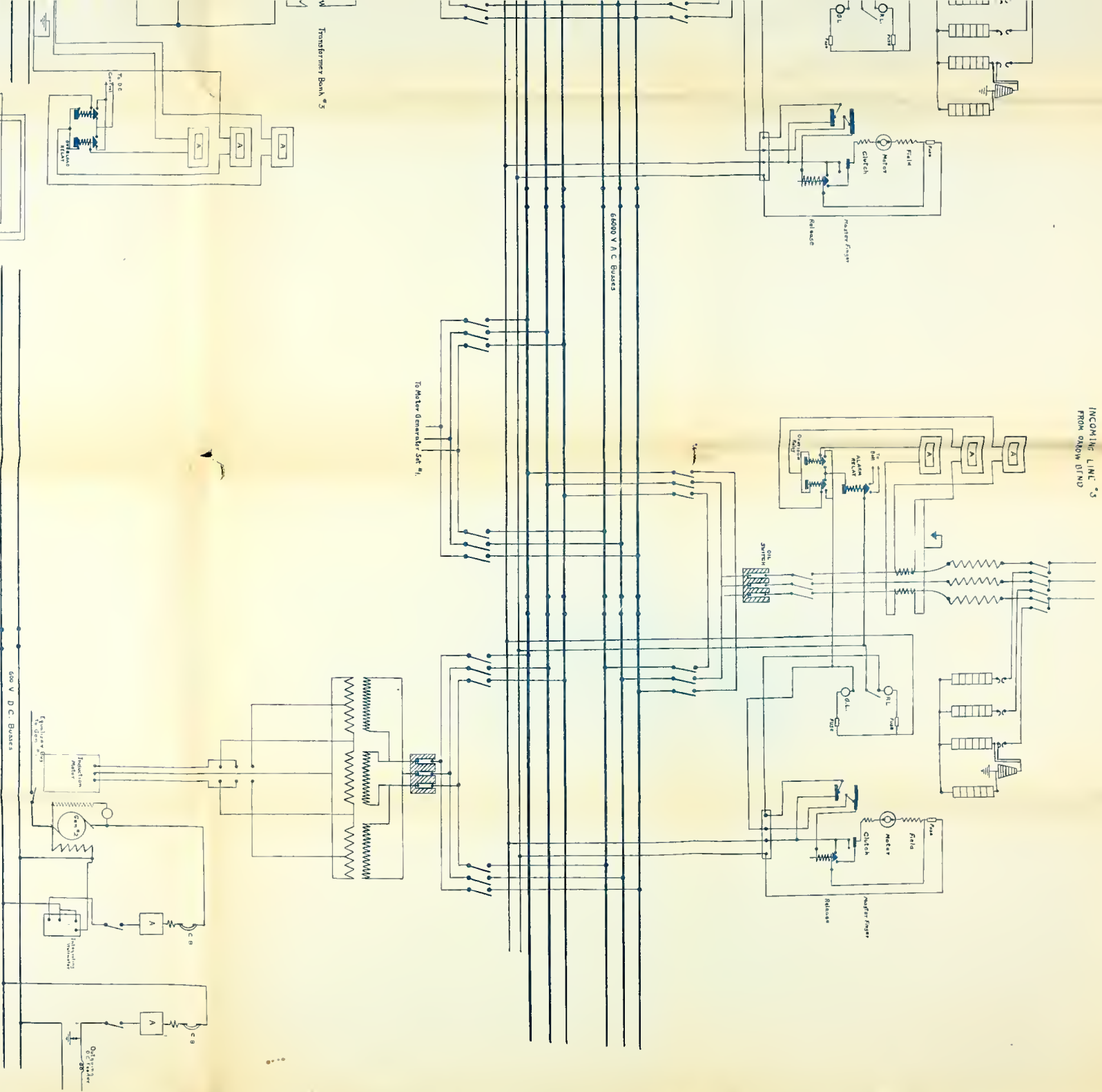
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3	1/2" X 1/2"	C
4	1/2" X 1/2"	D
5	1/2" X 1/2"	E
6	1/2" X 1/2"	F
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9	1/2" X 1/2"	I
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13	1/2" X 1/2"	M
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22	1/2" X 1/2"	V
23	1/2" X 1/2"	W
24	1/2" X 1/2"	X
25	1/2" X 1/2"	Y
26	1/2" X 1/2"	Z
27	1/2" X 1/2"	AA
28	1/2" X 1/2"	AB
29	1/2" X 1/2"	AC
30	1/2" X 1/2"	AD
31	1/2" X 1/2"	AE
32	1/2" X 1/2"	AF
33	1/2" X 1/2"	AG
34	1/2" X 1/2"	AH
35	1/2" X 1/2"	AI
36	1/2" X 1/2"	AJ
37	1/2" X 1/2"	AK
38	1/2" X 1/2"	AL
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40	1/2" X 1/2"	AN
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177	1/2" X 1/2"	FU
178	1/2" X 1/2"	FV
179	1/2" X 1/2"	FW
180	1/2" X 1/2"	FX
181	1/2" X 1/2"	FY
182	1/2" X 1/2"	FZ
183	1/2" X 1/2"	GA
184	1/2" X 1/2"	GB
185	1/2" X 1/2"	GC
186	1/2" X 1/2"	GD
187	1/2" X 1/2"	GE
188	1/2" X 1/2"	GF
189	1/2" X 1/2"	GG
190	1/2" X 1/2"	GH
191	1/2" X 1/2"	GI
192	1/2" X 1/2"	GJ
193	1/2" X 1/2"	GK
194	1/2" X 1/2"	GL
195	1/2" X 1/2"	GM
196	1/2" X 1/2"	GN
197	1/2" X 1/2"	GO
198	1/2" X 1/2"	GP
199	1/2" X 1/2"	GQ
200	1/2" X 1/2"	GR
201	1/2" X 1/2"	GS
202	1/2" X 1/2"	GT
203	1/2" X 1/2"	GU
204	1/2" X 1/2"	GV
205	1/2" X 1/2"	GW
206	1/2" X 1/2"	GX
207	1/2" X 1/2"	GY
208	1/2" X 1/2"	GZ
209	1/2" X 1/2"	HA
210	1/2" X 1/2"	HB
211	1/2" X 1/2"	HC
212	1/2" X 1/2"	HD
213	1/2" X 1/2"	HE
214	1/2" X 1/2"	HF
215	1/2" X 1/2"	HG
216	1/2" X 1/2"	HH
217	1/2" X 1/2"	HI
218	1/2" X 1/2"	HJ
219	1/2" X 1/2"	HK
220	1/2" X 1/2"	HL
221	1/2" X 1/2"	HM
222	1/2" X 1/2"	HN
223	1/2" X 1/2"	HO
224	1/2" X 1/2"	HP
225	1/2" X 1/2"	HQ
226	1/2" X 1/2"	HR
227	1/2" X 1/2"	HS
228	1/2" X 1/2"	HT
229	1/2" X 1/2"	HU
230	1/2" X 1/2"	HV
231	1/2" X 1/2"	HW
232	1/2" X 1/2"	HX
233	1/2" X 1/2"	HY
234	1/2" X 1/2"	HZ
235	1/2" X 1/2"	IA
236	1/2" X 1/2"	IB
237	1/2" X 1/2"	IC
238	1/2" X 1/2"	ID
239	1/2" X 1/2"	IE
240	1/2" X 1/2"	IF
241	1/2" X 1/2"	IG
242	1/2" X 1/2"	IH
243	1/2" X 1/2"	II
244	1/2" X 1/2"	IJ
245	1/2" X 1/2"	IK
246	1/2" X 1/2"	IL
247	1/2" X 1/2"	IM
248	1/2" X 1/2"	IN
249	1/2" X 1/2"	IO
250	1/2" X 1/2"	IP
251	1/2" X 1/2"	IQ
252	1/2" X 1/2"	IR
253	1/2" X 1/2"	IS
254	1/2" X 1/2"	IT
255	1/2" X 1/2"	IU
256	1/2" X 1/2"	IV
257	1/2" X 1/2"	IW
258	1/2" X 1/2"	IX
259	1/2" X 1/2"	IY
260	1/2" X 1/2"	IZ
261	1/2" X 1/2"	JA
262	1/2" X 1/2"	JB
263	1/2" X 1/2"	JC
264	1/2" X 1/2"	JD
265	1/2" X 1/2"	JE
266	1/2" X 1/2"	JF
267	1/2" X 1/2"	JG
268	1/2" X 1/2"	JH
269	1/2" X 1/2"	JI
270	1/2" X 1/2"	JJ
271	1/2" X 1/2"	JK
272	1/2" X 1/2"	JL
273	1/2" X 1/2"	JM
274	1/2" X 1/2"	JN
275	1/2" X 1/2"	JO
276	1/2" X 1/2"	JP
277	1/2" X 1/2"	JQ
278	1/2" X 1/2"	JR
279	1/2" X 1/2"	JS
280	1/2" X 1/2"	JT
281	1/2" X 1/2"	JU
282	1/2" X 1/2"	JV
283	1/2" X 1/2"	JW
284	1/2" X 1/2"	JX
285	1/2" X 1/2"	JY
286	1/2" X 1/2"	JZ
287	1/2" X 1/2"	KA
288	1/2" X 1/2"	KB
289	1/2" X 1/2"	KC
290	1/2" X 1/2"	KD
291	1/2" X 1/2"	KE
292	1/2" X 1/2"	KF
293	1/2" X 1/2"	KG
294	1/2" X 1/2"	KH
295	1/2" X 1/2"	KI
296	1/2" X 1/2"	KJ
297	1/2" X 1/2"	KK
298	1/2" X 1/2"	KL
299	1/2" X 1/2"	KM
300	1/2" X 1/2"	KN
301	1/2" X 1/2"	KO
302	1/2" X 1/2"	KP
303	1/2" X 1/2"	KQ
304	1/2" X 1/2"	KR
305	1/2" X 1/2"	KS
306	1/2" X 1/2"	KT
307	1/2" X 1/2"	KU
308	1/2" X 1/2"	KV
309	1/2" X 1/2"	KW
310	1/2" X 1/2"	KX
311	1/2" X 1/2"	KY
312	1/2" X 1/2"	KZ
313	1/2" X 1/2"	LA
314	1/2" X 1/2"	LB
315	1/2" X 1/2"	LC
316	1/2" X 1/2"	LD
317	1/2" X 1/2"	LE
318	1/2" X 1/2"	LF
319	1/2" X 1/2"	LG
320	1/2" X 1/2"	LH
321	1/2" X 1/2"	LI
322	1/2" X 1/2"	LJ
323	1/2" X 1/2"	LK
324	1/2" X 1/2"	LL
325	1/2" X 1/2"	LM
326	1/2" X 1/2"	LN
327	1/2" X 1/2"	LO
328	1/2" X 1/2"	LP
329	1/2" X 1/2"	LQ
330	1/2" X 1/2"	LR
331	1/2" X 1/2"	LS
332	1/2" X 1/2"	LT
333	1/2" X 1/2"	LU
334	1/2" X 1/2"	LV
335	1/2" X 1/2"	LW
336	1/2" X 1/2"	LX
337	1/2" X 1/2"	LY
338	1/2" X 1/2"	LZ
339	1/2" X 1/2"	MA
340	1/2" X 1/2"	MB
341	1/2" X 1/2"	MC
342	1/2" X 1/2"	MD
343	1/2" X 1/2"	ME
344	1/2" X 1/2"	MF
345	1/2" X 1/2"	MG
346	1/2" X 1/2"	MH
347	1/2" X 1/2"	MI
348	1/2" X 1/2"	MJ
349	1/2" X 1/2"	MK
350	1/2" X 1/2"	ML
351	1/2" X 1/2"	MM



**150 K.W.
SUBSTATION**
TO BE USED AT
NEAL, IDA.
SCALE $\frac{1}{4}"=1'$ MAY 15, 1910



INCOMING LINE "3"
FROM OAKW OFND



WIRING DIAGRAM FOR **SUBSTATION**

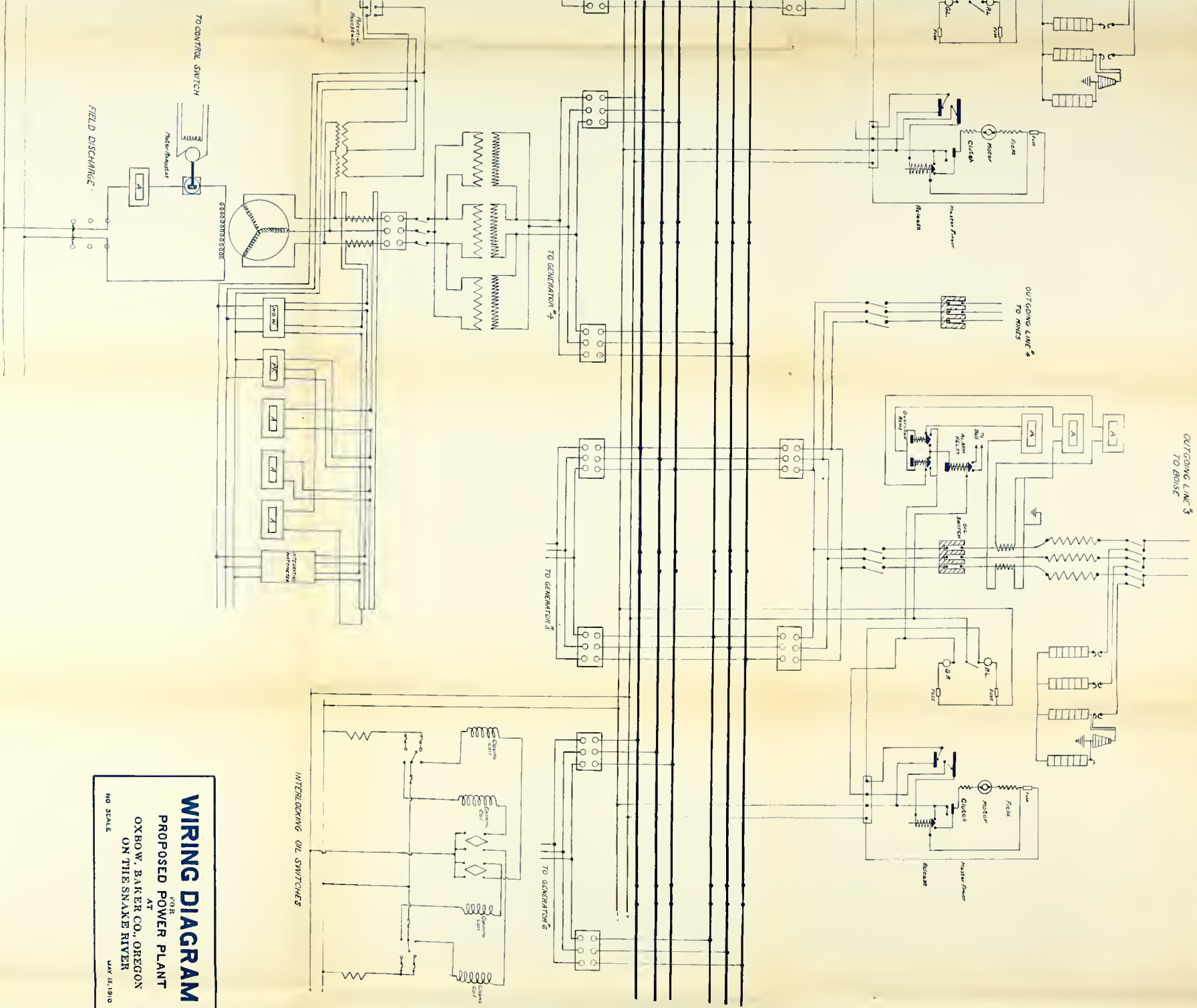
BRISE, IDAHP

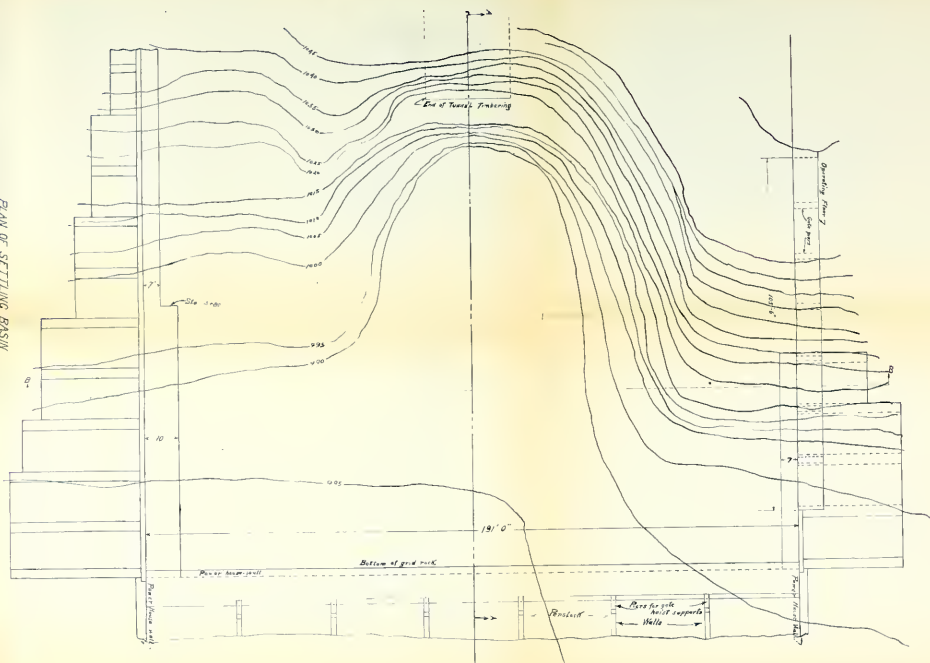
NO SCALE

MAY 15, 1910

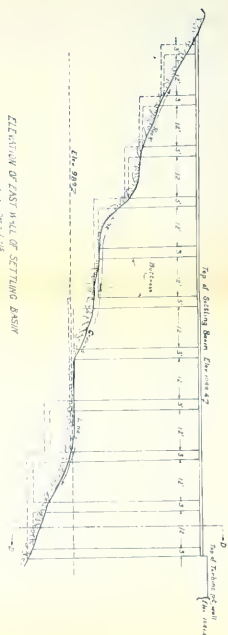
OUTGOING LINE 3
TO BOISE

OUTGOING LINE 4
TO MINES

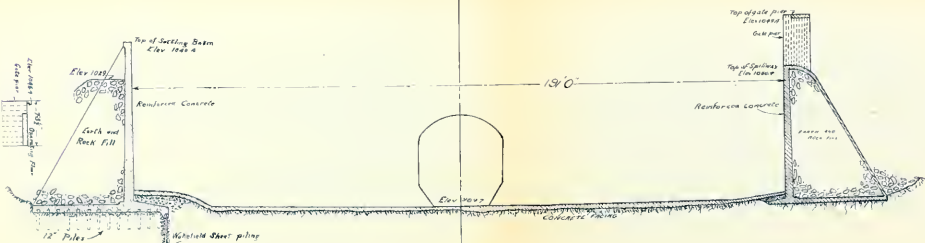




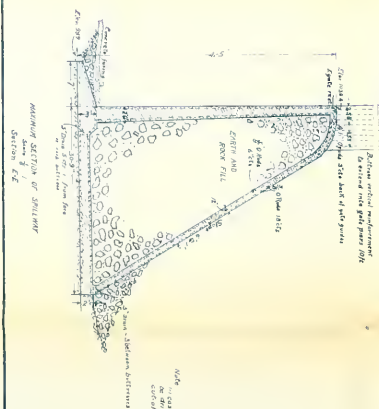
PLAN OF SETTLING BASIN
Scale 1"=15'



ELEVATION OF EAST WALL OF SETTLING BASIN

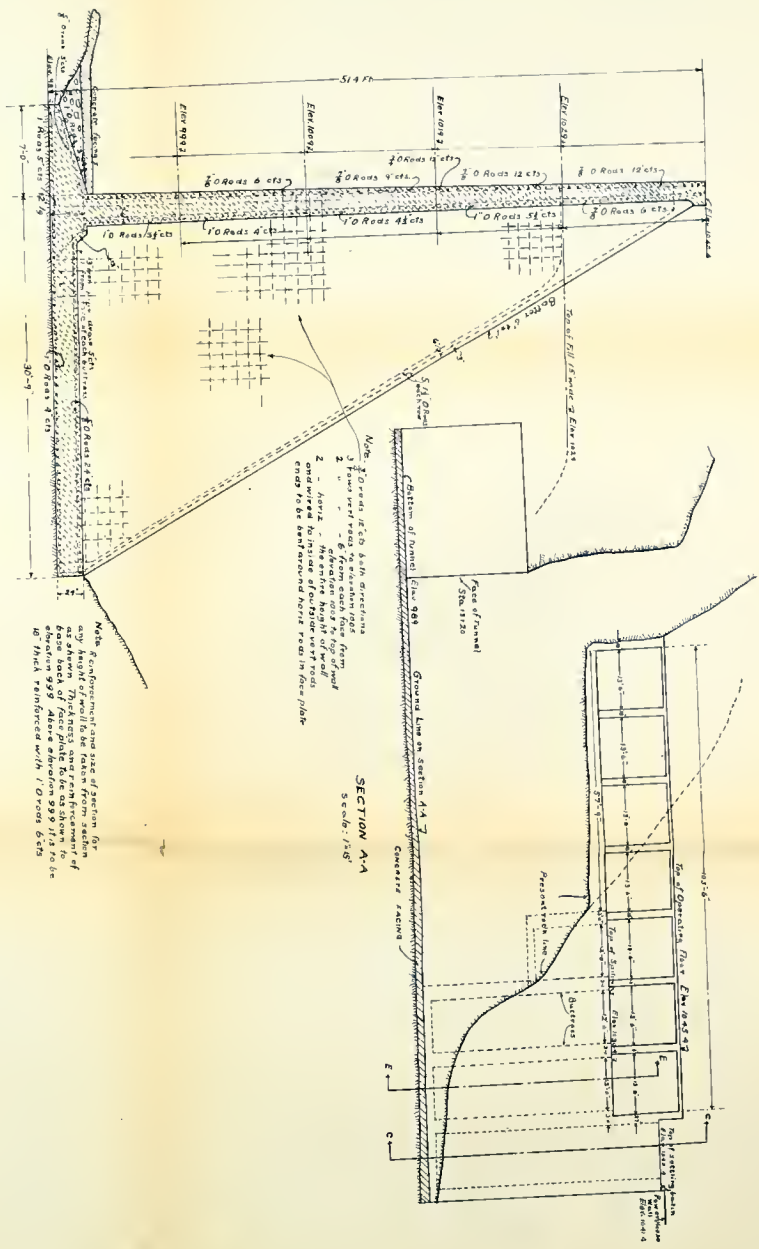


SECTION B-B
Scale 1"=15'

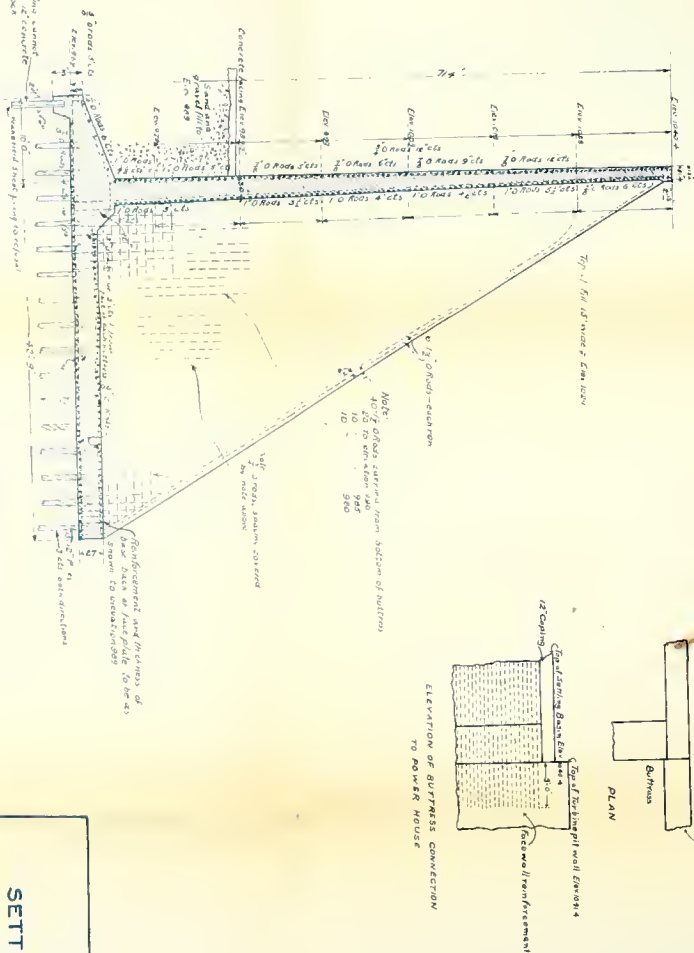


MAXIMUM SECTION OF SPILLWAY

Note — in case a heat piling cannot be drawn carry it over to next cut of wall to rock

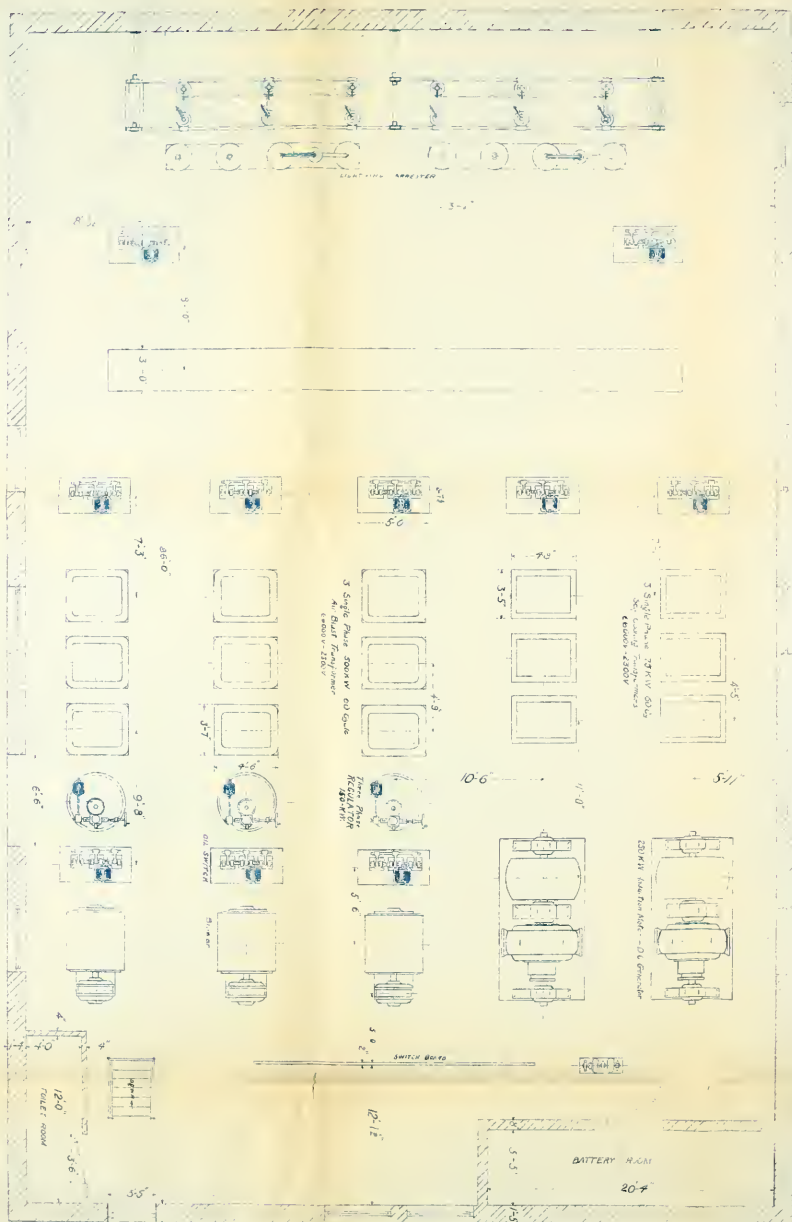


MAXIMUM SECTION OF SETTLING BASIN WALL IN ROCK
Scale 1/8" = 1'-0"
SECTION C-C



ELEVATION OF BUTTRESS CONNECTION
TO POWER HOUSE

DETAILS
OF
SETTLING BASIN
AT
PROPOSED POWER PLANT
OXBOW, BAKER CO. OREGON
ON THE SNAKE RIVER

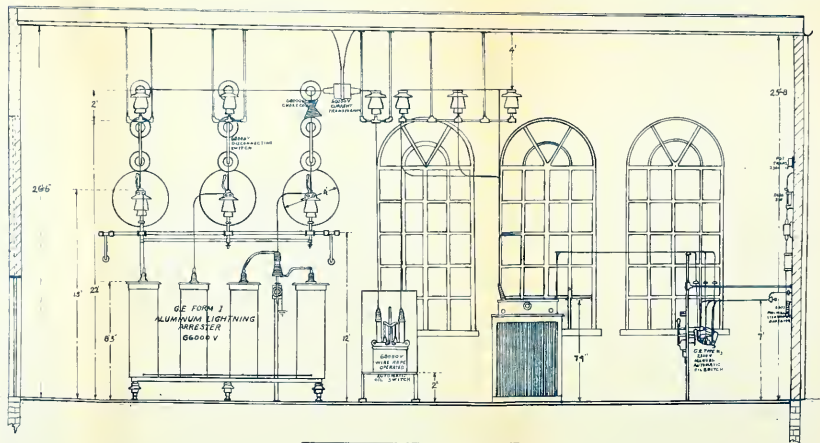
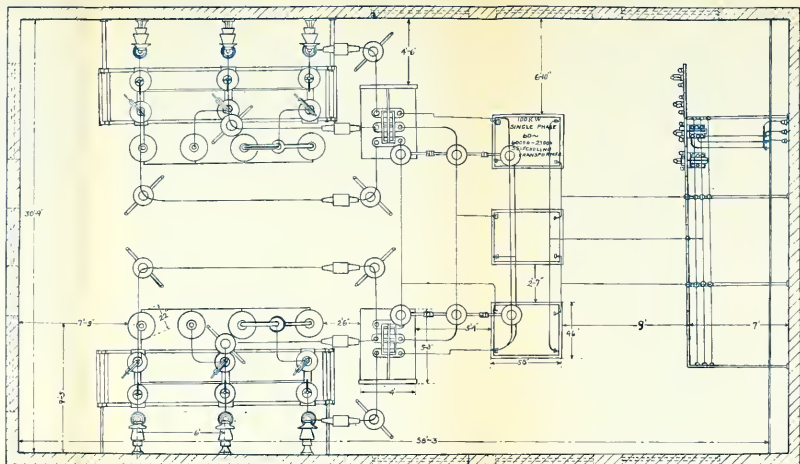




AIR CHAMBER

**5000 K.W.
SUBSTATION**
TO BE USED AT
BOISE IDA.

SCALE $\frac{1}{2}'' = 1'$ MAY 15, 1910



300 K.W. SUBSTATION

TO BE USED AT
PLYMOUTH, IDA.
EMMETT, IDA

SCALE $\frac{1}{2}''=1'$

MAY 15 1960

